

*Budapest University of Technology and Economics
Department of Structural Engineering*

Optimal stiffener geometry based on nonlinear analysis of longitudinally stiffened plates under compression

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Earthquake analysis of bridges in moderate seismic zones

In Hungary there is no tradition of seismic design

Hungary has been classified into moderate seismic zones

Damage can be significant

EUROCODE 8 : seismic safety check has to be done

Earthquake analysis of bridges in moderate seismic zones

1. Analysis of extant bridges

Database about extant bridges

Representative structures

Numerical analysis (spectral and time-history)

Critical structural types and details

Damage prediction



Alternative reinforcing/retrofit methods

2. New design conceptions

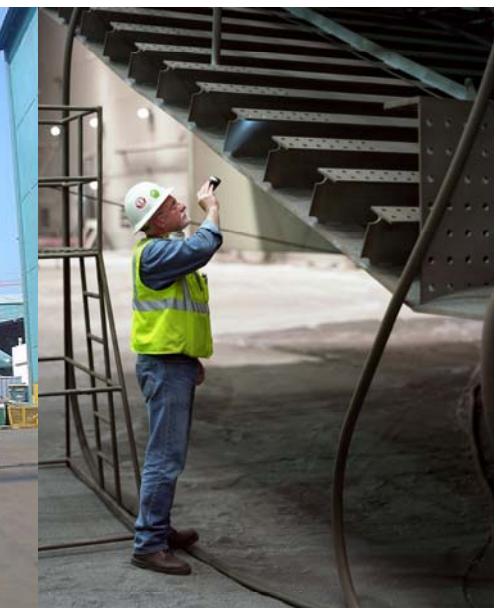
Stiffened plates

Thin-walled structures

Ultimate load/Material consumption
Stability failure is dominant

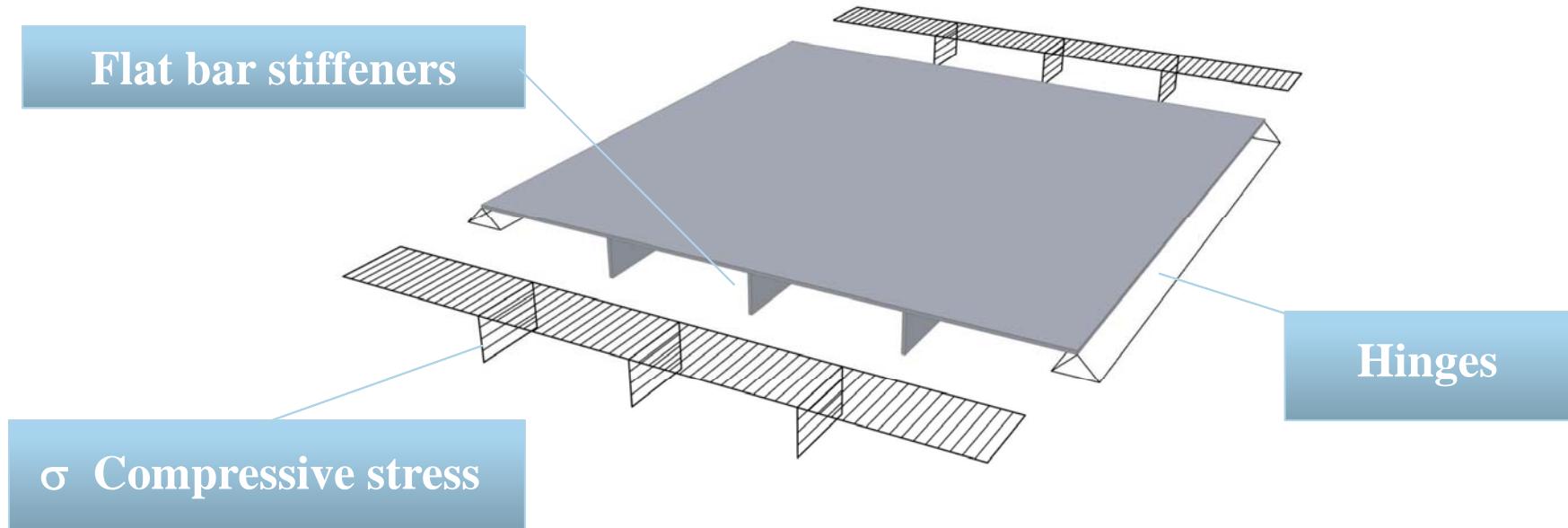
Ultimate load, failure

Stiffness ratios



Aim of the studies

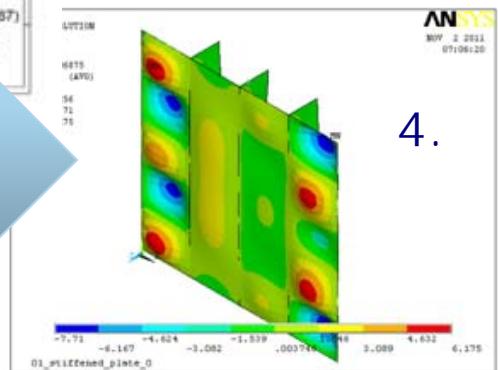
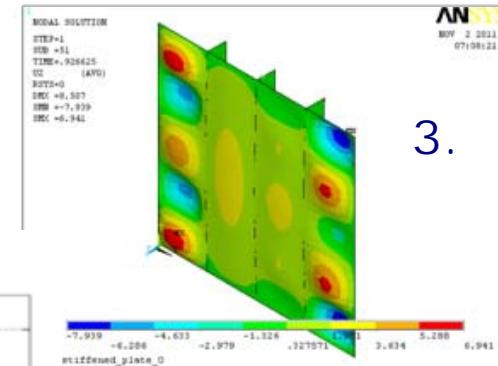
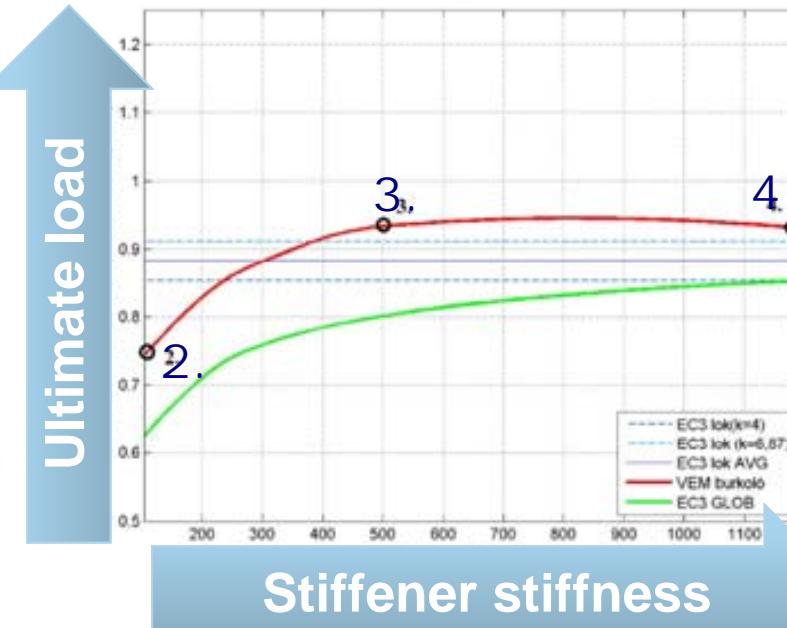
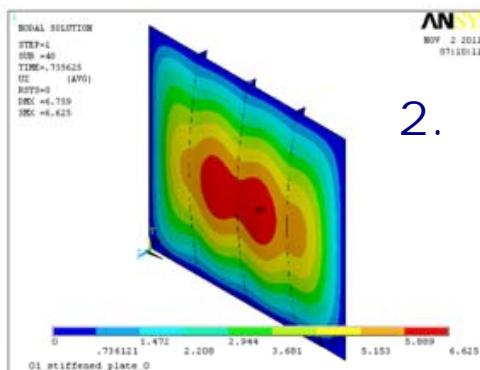
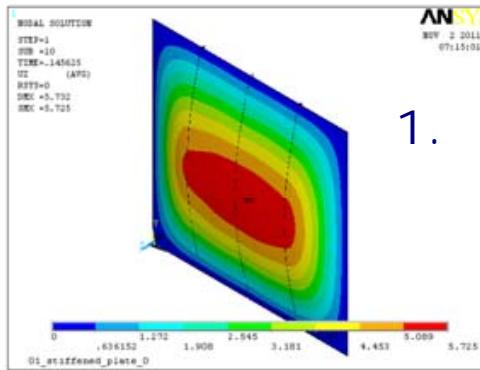
Longitudinally stiffened plate, pure compression.



Parameters characterizing the stiffness relations :

$$\gamma = \frac{EI_y}{D_p b}; \quad \delta = \frac{A_x}{bt_p}; \quad D_p = \frac{Et_p^3}{12(1-\nu^2)};$$

Optimal stiffener geometry



Methods to calculate the optimal relative stiffness

By elastic critical stress

- $$\gamma_c^* = \frac{1}{n_s + 1} \left[\frac{\alpha^2}{m_{ov}} \left(4(n_s + 1)^2 (1 + \delta(n_s + 1)) - 2 - \frac{\alpha^2}{m_{ov}} \right) - 1 \right]$$
- Perfect geometry, linear elastic material
- Not applicable directly for design

By ultimate loads \Rightarrow GMNI analysis

- Post-critical load bearing capacity
- Imperfections, non-linear, plastic material model
- Can be used for design directly

ULS according to EC3-1-5

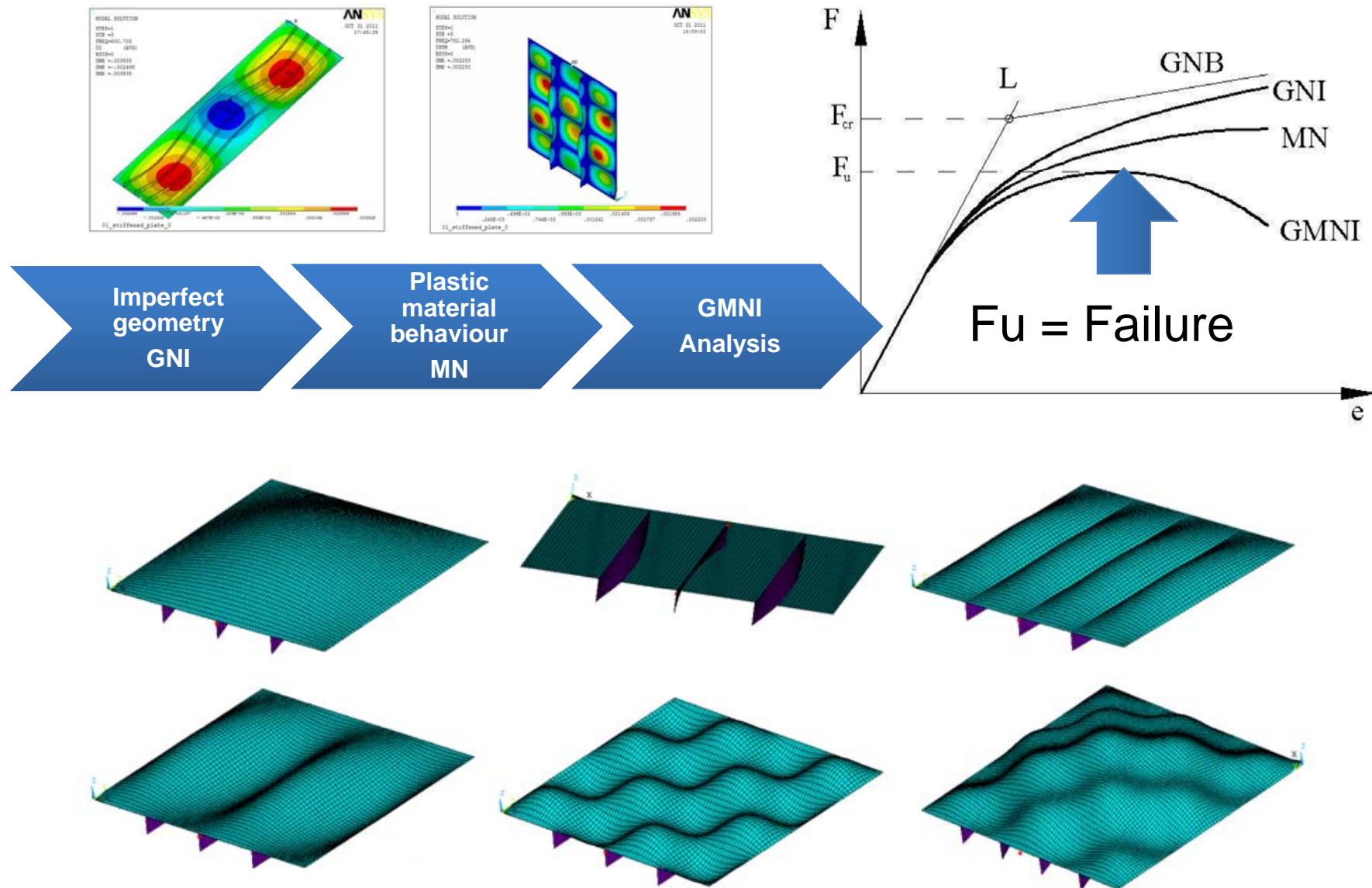
Effective cross-section method

It takes into account the buckling of the elements and their interactions.

Stability failures according to EC3-1-5

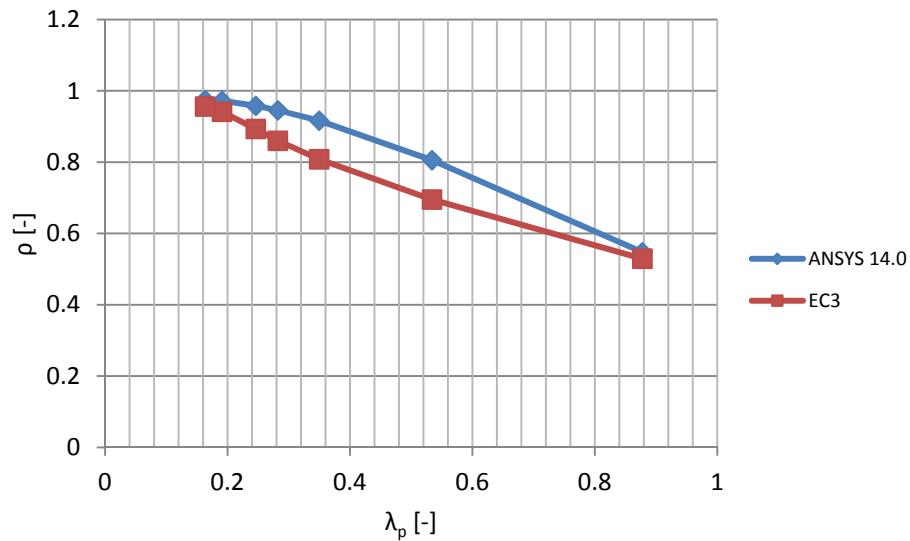
Local	Global	Stiffener
<ul style="list-style-type: none">Buckling of subpanels between the stiffeners	<ul style="list-style-type: none">Plate type behaviourColumn type buckling behaviourInteraction	<ul style="list-style-type: none">Distortional bucklingMinimum geometric requirementsShould be avoided!

Numerical model

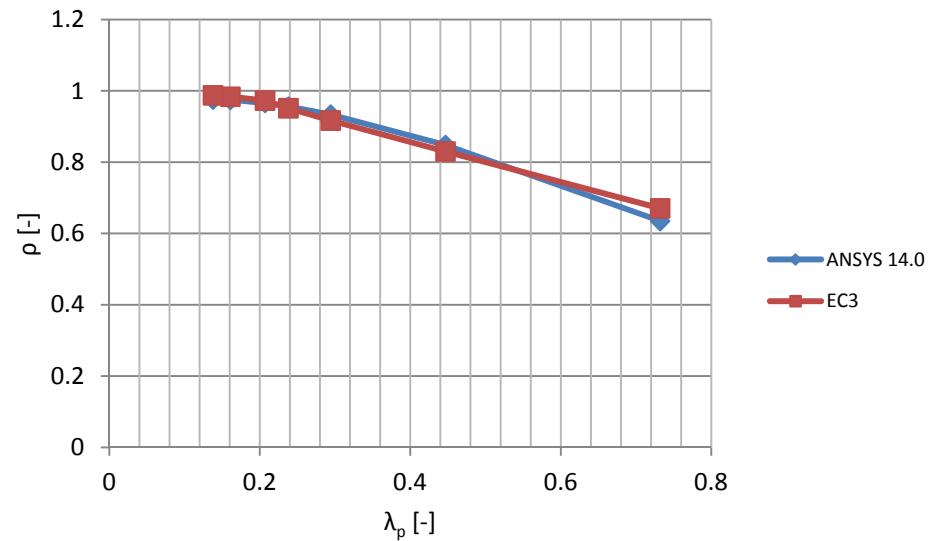


Results 1

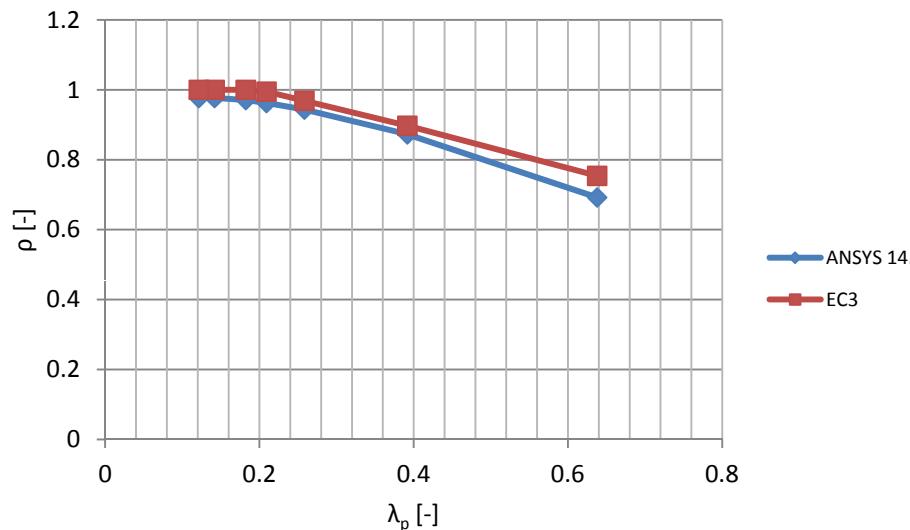
ρ reducing factor – plate slenderness



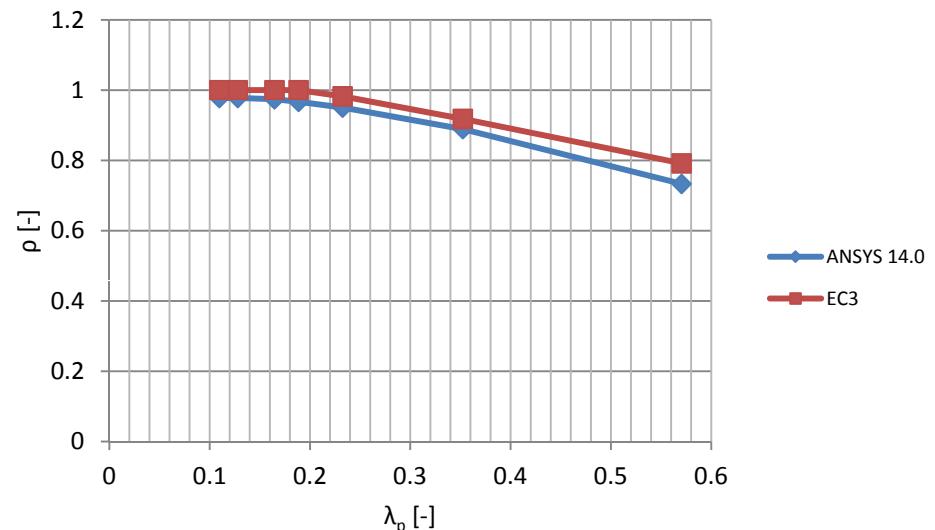
ρ reducing factor – plate slenderness



ρ reducing factor – plate slenderness

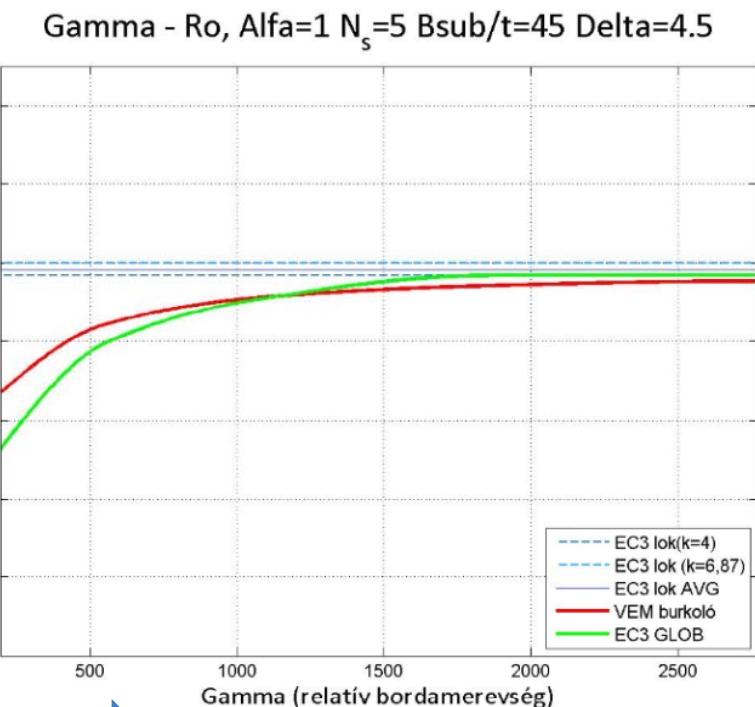
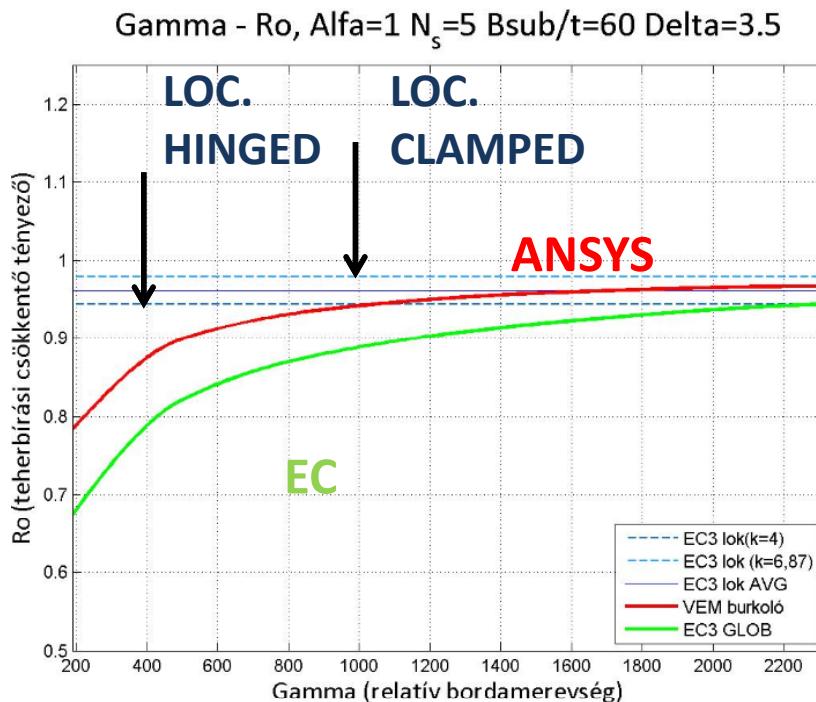


ρ reducing factor – plate slenderness



Results 2

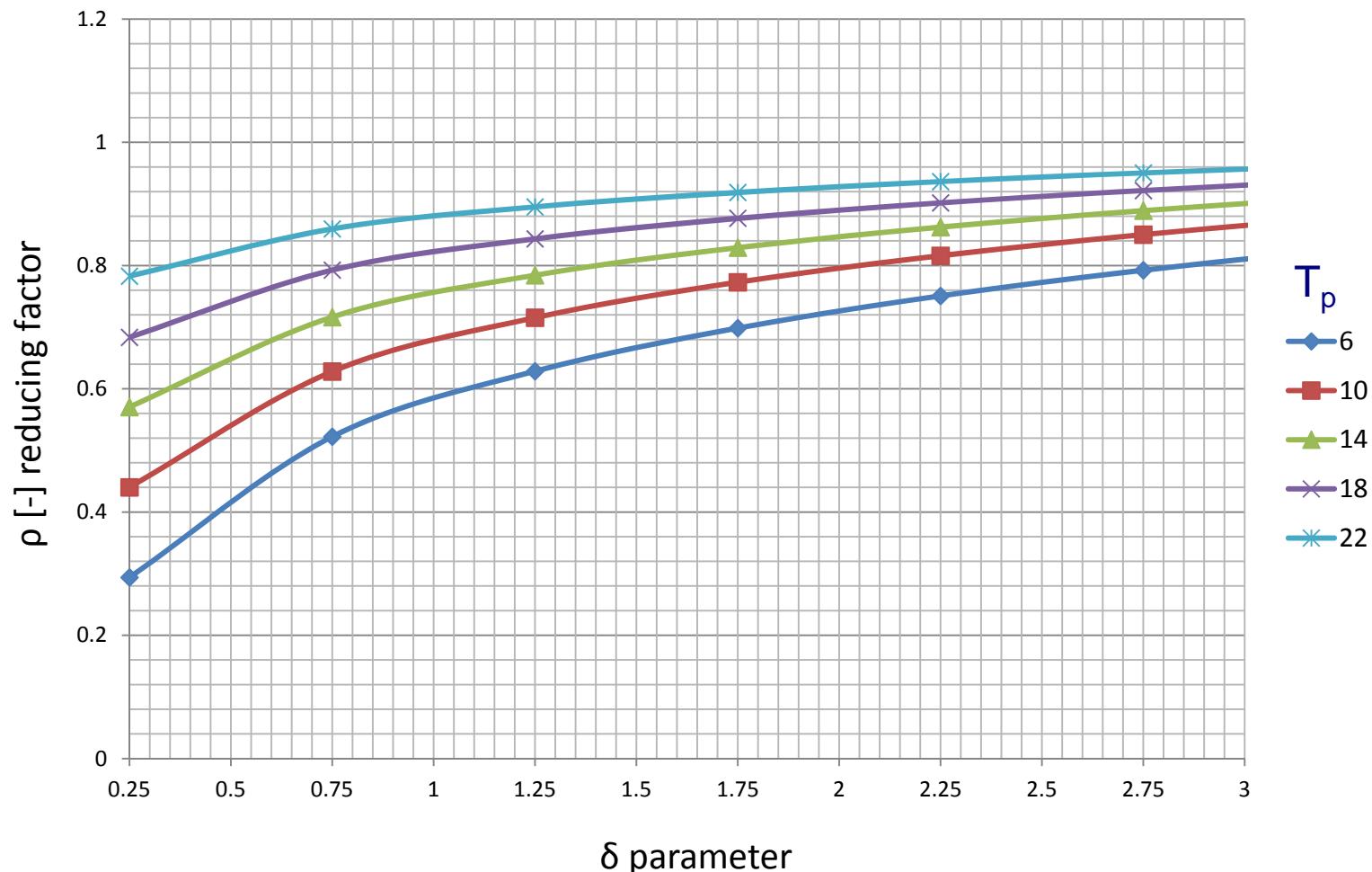
Plates with higher and lower slenderness



Stiffener stiffness

Results 3

ρ reducing factor – δ parameter ($\alpha = 1$, $ns = 3$, $b = 4000$)



Thank you for your attention!

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